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 221 East Fourth Street
 Suite 1150
 Cincinnati, Ohio 45202
 513.241.3222
 THPLTD.com

PROJECT
 CLEVELAND HOPKINS INTERNATIONAL AIRPORT

THP #
 24046.00

DRAWING NUMBER

TITLE
 JET BRIDGE FOUNDATIONS CRITERIA DESIGN

ISSUE
 100% DRAFT

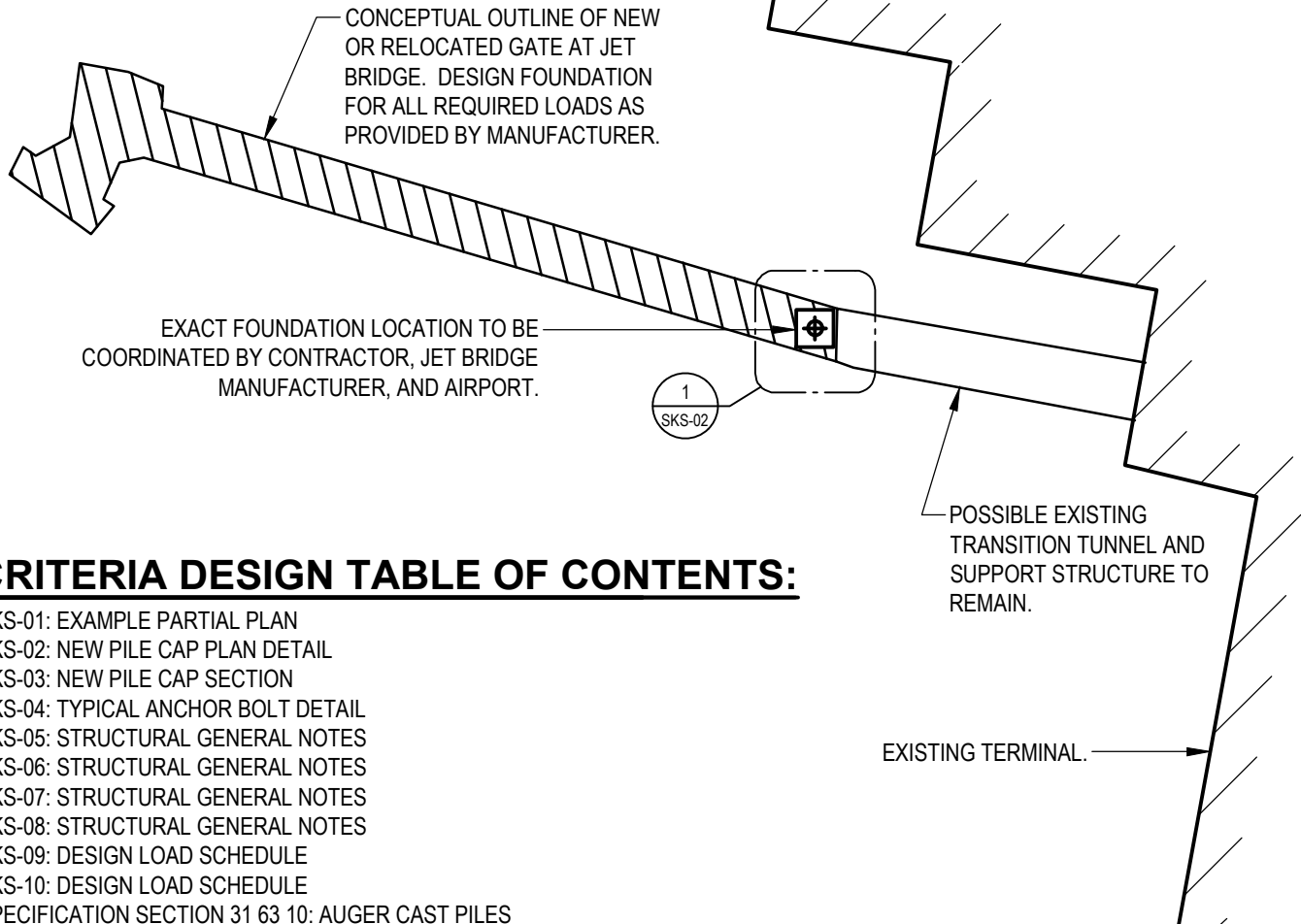
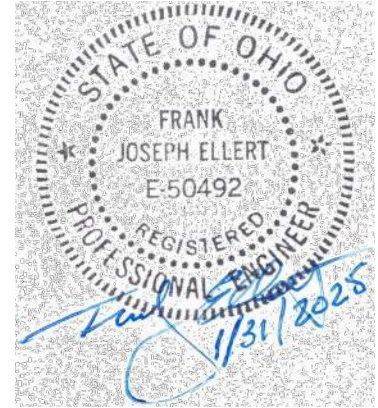
BY
 CAH

DATE
 1/31/2025

SKS
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NOTES:

1. EXAMPLE GATE A5 JETBRIDGE PARTIAL PLAN SHOWN FOR CONCEPTUAL PURPOSES ONLY. PROVIDE UPDATED PARTIAL PLAN FOR EACH INSTALLATION AS APPROPRIATE.
2. REFER TO SKS-05 THROUGH SKS-10 FOR STRUCTURAL GENERAL NOTES, INCLUDING CRITERIA DESIGN LOADS.
3. RECONFIRM ALL DESIGN LOADS AND PROVIDE FINAL DESIGN AND DETAILS MODIFIED OR REVISED AS APPROPRIATE, STAMPED BY AN ENGINEER LICENSED IN THE STATE OF OHIO.



CRITERIA DESIGN TABLE OF CONTENTS:

- SKS-01: EXAMPLE PARTIAL PLAN
- SKS-02: NEW PILE CAP PLAN DETAIL
- SKS-03: NEW PILE CAP SECTION
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- SKS-05: STRUCTURAL GENERAL NOTES
- SKS-06: STRUCTURAL GENERAL NOTES
- SKS-07: STRUCTURAL GENERAL NOTES
- SKS-08: STRUCTURAL GENERAL NOTES
- SKS-09: DESIGN LOAD SCHEDULE
- SKS-10: DESIGN LOAD SCHEDULE
- SPECIFICATION SECTION 31 63 10: AUGER CAST PILES



EXAMPLE JET BRIDGE
PARTIAL PLAN

1" = 20'-0"



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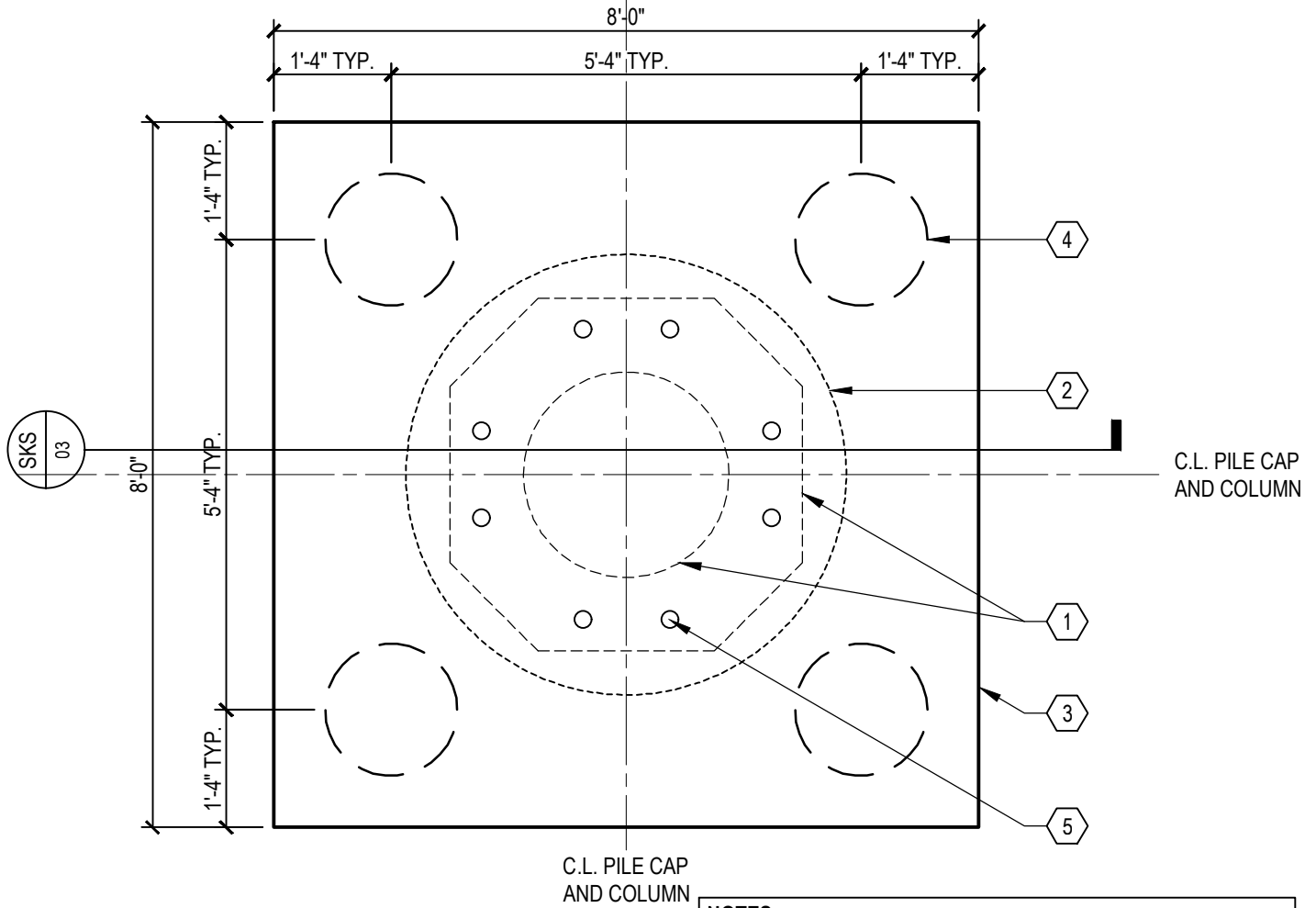
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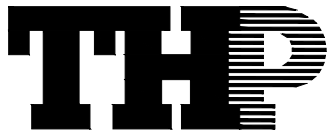


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- ① COLUMN AND BASE PLATE BY OTHERS.
- ② EXISTING 60" DIA. DRILLED PIER. DEMO TO 1'-0" BELOW BOTTOM OF NEW PILE CAP ELEVATION.
- ③ 4'-0" THICK PILE CAP WITH (10) #9 EPOXY COATED EACH WAY, TOP AND BOTTOM.
- ④ (4) 18" DIA. AUGERCAST PILES WITH 40 TONS COMPRESSION AND 10 TONS TENSION ALLOWABLE CAPACITY. FINAL PILE DESIGN AND LENGTH TO BE DETERMINED PER THE PILE CONTRACTOR'S PROFESSIONAL ENGINEER.
- ⑤ (8) 2-1/4" DIA. ANCHOR BOLTS. REFER TO TYPICAL ANCHOR BOLT DETAIL SKS-04 FOR MORE INFORMATION. COORDINATE EXACT SIZE AND LOCATIONS WITH COLUMN AND BASE PLATE SUPPLIER.

NEW PILE CAP
SKS
PLAN DETAIL
 02 1/2" = 1'-0"



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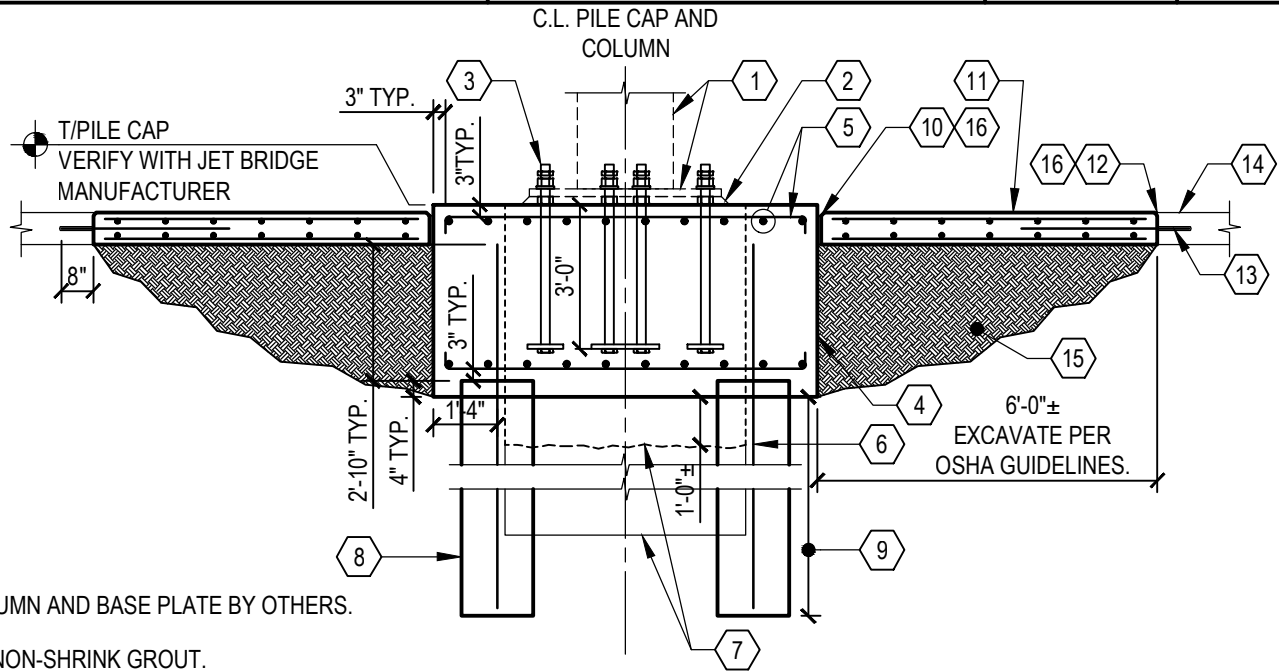
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- 1 COLUMN AND BASE PLATE BY OTHERS.
- 2 2"± NON-SHRINK GROUT.
- 3 2-1/4" DIA. ANCHOR BOLTS WITH 10" PROJECTION. REFER TO TYPICAL ANCHOR BOLT DETAIL SKS-04 FOR MORE INFORMATION. COORDINATE EXACT SIZE AND LOCATIONS WITH COLUMN AND BASE PLATE SUPPLIER.
- 4 4'-0" THICK PILE CAP. FORM PILE CAP SIDES AND BACKFILL PER KEY NOTE 15.
- 5 (10) #9 EPOXY COATED EACH WAY, TOP AND BOTTOM. ALL BARS TO BE HOOKED EACH END.
- 6 (1) #9 FULL LENGTH PLACED IN CENTER OF PILE, TYPICAL.
- 7 POSSIBLE EXISTING FOUNDATION, UTILITY BANK, OR OTHER OBSTRUCTION. REMOVE FOUNDATIONS TO ALLOW 1'-0" CLEAR AS SHOWN AND FILL VOID SPACE WITH CONCRETE, OR MODIFY FINAL DESIGN OR RELOCATE OTHER OBSTRUCTIONS AS APPROPRIATE.
- 8 18" DIA. AUGERCAST PILE WITH 40 TONS COMPRESSION AND 10 TONS TENSION ALLOWABLE CAPACITY, TYPICAL. FINAL PILE DESIGN AND LENGTH TO BE DETERMINED PER THE PILE CONTRACTOR'S PROFESSIONAL ENGINEER.
- 9 APPROXIMATE LENGTH 35'-0"±. IGNORE THE SOIL BOND ABOVE THE EXISTING DRILLED PIER BEARING ELEVATION. FINAL LENGTH TO BE DETERMINED PER THE PILE CONTRACTOR'S PROFESSIONAL ENGINEER.
- 10 1/2" ISOLATION JOINT FULL DEPTH AT NEW SLAB ON GRADE.
- 11 NEW SLAB ON GRADE. MATCH EXISTING SLAB ON GRADE THICKNESS (8" THICK MINIMUM), REINFORCE WITH #5 EPOXY COATED AT 12" O.C. EACH WAY, TOP AND BOTTOM.
- 12 SAWCUT EXISTING SLAB ON GRADE FULL DEPTH TO INSTALL NEW PILE CAP.
- 13 #5 X 3'-6" EPOXY COATED DOWELS AT 12" O.C. AT PERIMETER OF NEW SLAB ON GRADE. DRILL AND EPOXY EMBED 8" INTO EXISTING SLAB ON GRADE AT MID-DEPTH. USE HILTI HIT-HY 200 ADHESIVE.
- 14 EXISTING SLAB ON GRADE TO REMAIN.
- 15 BACKFILL EXCAVATION WITH CONTROLLED DENSITY FILL.
- 16 TOOL JOINT AND PROVIDE URETHANE SEALANT.

NOTES:

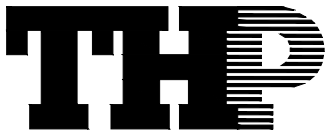
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**NEW PILE CAP
 SECTION**

03

1/4" = 1'-0"



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04**

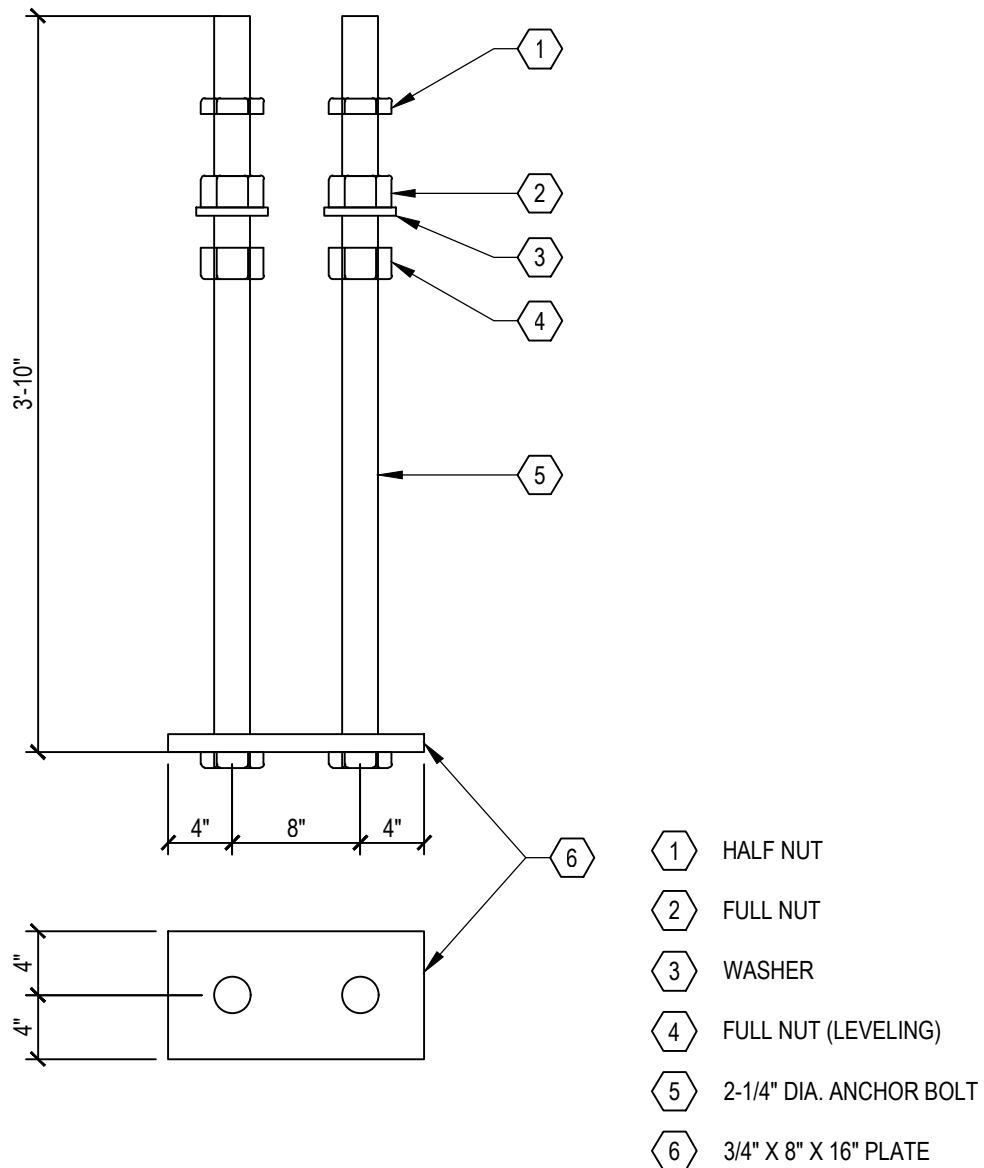
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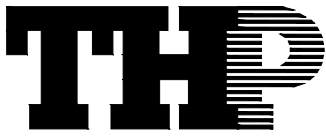


TYPICAL ANCHOR BOLT

DETAIL



NO SCALE



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STRUCTURAL GENERAL NOTES:

A. CODES AND SPECIFICATIONS

1. 2024 OHIO BUILDING CODE.
2. ASCE/SEI 7-16 WITH SUPPLEMENT 1, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.
3. ACI 301-20, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS AS MODIFIED BY THE CONSTRUCTION DOCUMENTS.
4. ANSI/AISC 303-22, CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES AS MODIFIED BY THE CONSTRUCTION DOCUMENTS.
5. ANSI/AWS D1.1, STRUCTURAL WELDING CODE - STEEL.
6. ADDITIONAL TECHNICAL SPECIFICATIONS IN THE PROJECT MANUAL.

B. FOUNDATIONS

1. FOUNDATION ELEVATIONS SHOWN ARE ESTIMATED AND ARE FOR BIDDING ONLY. ACTUAL ELEVATIONS MAY VARY TO SUIT SUBSURFACE SOIL CONDITIONS.
2. ALL BEARING SURFACES SHALL BE UNDISTURBED, LEVEL (WITHIN 1 IN 12), AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
3. ALL FOOTINGS, CAPS AND GRADE BEAMS ARE TO BE POURED NEAT AGAINST EARTH BANKS (WITHOUT SIDE FORMS) UNLESS APPROVED OTHERWISE BY BOTH THE GEOTECHNICAL AND STRUCTURAL ENGINEERS. WHERE EARTH CUTS WILL NOT STAND, SIDES SHALL BE FORMED AND BACKFILLED WITH COMPACTED MATERIAL, SUBJECT TO ENGINEERS' APPROVAL.
4. PILE FOUNDATIONS ARE DESIGNED FOR 18" DIAMETER, 40 TON COMPRESSION AND 10 TON TENSION ALLOWABLE CAPACITY, AUGERED, CAST-IN-PLACE GROUTED PILES. UNLESS NOTED OTHERWISE, REINFORCE ALL PILES PER DETAIL 2/S2. APPROXIMATE PILE LENGTH = 35'-0" ±. FINAL PILE DESIGN AND PILE LENGTH TO BE DETERMINED PER THE PILE CONTRACTOR'S PROFESSIONAL ENGINEER. PRIOR TO PILE INSTALLATION, CONTRACTOR TO SUBMIT COMPLETE DESIGN CALCULATIONS AND ALL WORKING DRAWINGS SIGNED AND SEALED BY THE PILE CONTRACTOR'S PROFESSIONAL ENGINEER. REFER TO SPECIFICATION SECTION 31 63 10 FOR ADDITIONAL INFORMATION. VERIFY PILE LENGTH/TIP ELEVATION OF EACH PILE WITH THE GEOTECHNICAL ENGINEER PRIOR TO GROUTING.
5. SET COLUMN DOWELS AND ANCHOR RODS WITH A TEMPLATE PRIOR TO PLACING CONCRETE.



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STRUCTURAL GENERAL NOTES:

C. CONCRETE

1. CONCRETE STRENGTHS:

- a. EXTERIOR CONCRETE EXPOSED TO WEATHER: 5000 PSI AIR ENTRAINED (6% ±1%).
- b. BACKFILL (LEAN) CONCRETE: 1500 PSI.

2. PROVIDE 3/4" BEVELS AT EDGES OF EXPOSED SLABS AND TOP EDGES AND CORNERS OF EXPOSED PILE CAPS.

3. JOINTS NOT INDICATED ON STRUCTURAL DRAWINGS ARE NOT PERMITTED UNLESS APPROVED BY THE STRUCTURAL ENGINEER.

4. CONCRETE CONSTRUCTION TOLERANCES SHALL BE PER ACI.

D. REINFORCING STEEL

1. ALL REINFORCING BARS: 60 KSI YIELD.

2. REINFORCE ALL SLABS PER DRAWINGS.

3. PROVIDE TENSION SPLICES UNLESS NOTED OTHERWISE.

4. PLACE WITH MINIMUM CLEAR COVER BETWEEN REINFORCING STEEL AND CONCRETE SURFACES AS SHOWN. IF NOT SHOWN, PROVIDE CLEAR COVER PER ACI. ALL REINFORCING SHALL BE PLACED AS SHOWN AND DETAILED, AND WITHIN ACI TOLERANCE.

5. EPOXY COAT REINFORCING WHERE INDICATED.

E. STRUCTURAL STEEL


1. ALL WORK SHALL CONFORM TO AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) STANDARDS AND THE PROJECT SPECIFICATIONS.

2. MATERIAL:

- a. PLATES: ASTM A572 GRADE 50 (Fy 50 KSI).
- b. ANCHOR RODS: ASTM F1554 (Fy 55 KSI), U.N.O.

3. ALL WELDING MATERIALS, WELDING PROCEDURES AND OPERATORS PERFORMING WELDING TO BE QUALIFIED PER AWS D1.1.

4. ANCHOR BOLTS AND PLATES SHALL BE HOT DIPPED GALVANIZED.

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STRUCTURAL GENERAL NOTES:

F. COORDINATION AND CONSTRUCTION

1. THE CONTRACTOR SHALL COMPLETE ALL WORK REQUIRED AND NECESSARY FOR THE PROJECT IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, DRAWINGS, AND REFERENCED STANDARDS. THE SPECIFICATIONS AND DRAWINGS COMPLEMENT EACH OTHER, THE CONTRACTOR SHALL THOROUGHLY REVIEW BOTH BEFORE PROCEEDING WITH ANY WORK.
2. UNLESS OTHERWISE NOTED, DETAILS, SECTIONS AND NOTES ON THE STRUCTURAL DRAWINGS ARE INTENDED TO APPLY TO SIMILAR SITUATIONS ELSEWHERE.
3. SHOP DRAWINGS AND INSTALLATION DRAWINGS SHALL BE PREPARED AND SUBMITTED FOR REVIEW. SUBMITTALS PREPARED BY SUPPLIERS AND SUBCONTRACTORS SHALL BE REVIEWED BY THE TRADE CONTRACTOR AND GENERAL CONTRACTOR OR CONSTRUCTION MANAGER PRIOR TO SUBMITTING TO ARCHITECT/ENGINEER.
 - a. FIELD VERIFY ALL EXISTING DIMENSIONS, ELEVATIONS AND CONDITIONS WHICH AFFECT FABRICATION AND SHOW ON SHOP DRAWINGS.
 - b. SUBMIT COMPLETE SHOP DRAWINGS WITH MANUFACTURERS' DATA, ETC. SHOW ALL CONNECTIONS AND DETAILS NECESSARY TO FULLY DESCRIBE AND PROPERLY INSTALL THE WORK.
 - c. STRUCTURAL ENGINEER'S REVIEW SHALL BE FOR GENERAL ARRANGEMENT AND CONFORMANCE WITH THE STRUCTURAL INTENT ONLY.
4. THE SPECIFICATIONS AND STRUCTURAL DRAWINGS TYPICALLY REFER TO THE FINISHED STRUCTURE. UNLESS NOTED OTHERWISE, THEY DO NOT PRESCRIBE THE METHOD OF CONSTRUCTION.
5. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO WORKING HOURS.
6. THE ARCHITECT'S AND ENGINEER'S OBSERVATION AND REVIEW OF CONTRACTORS' PERFORMANCE DOES NOT INCLUDE REVIEW OF ADEQUACY OF CONTRACTOR'S SAFETY MEASURES IN, ON, OR NEAR THE CONSTRUCTION SITE.

G. UTILITIES

1. PRIOR TO EXCAVATION AND EARTHWORK, VERIFY LOCATIONS OF UNDERGROUND UTILITIES WITH THE UTILITY COMPANIES, CONSTRUCTION MANAGER, AND THE OWNER. EXCAVATE OR SURVEY TO ESTABLISH EXACT UTILITY LOCATIONS. UTILITY LOCATIONS IF SHOWN ON THE CONTRACT DRAWINGS ARE ONLY APPROXIMATE AND CANNOT BE USED TO ASSURE THE CONTRACTOR OF ADEQUATE CLEARANCE.
2. ALL UTILITIES SHALL BE ADEQUATELY PROTECTED FROM DAMAGE. WHERE UTILITIES ARE ENCOUNTERED, CONTRACTOR SHALL IMMEDIATELY NOTIFY THE UTILITY COMPANY, CONSTRUCTION MANAGER, AND THE OWNER BEFORE PROCEEDING. ACTIVE UTILITIES ENCOUNTERED SHALL BE PROTECTED, SUPPORTED, OR RELOCATED AS DIRECTED. INACTIVE AND ABANDONED UTILITIES SHALL BE REMOVED, PLUGGED, OR CAPPED AS DIRECTED.
3. CALL THE OHIO UTILITIES PROTECTION SERVICE TOLL FREE 1-800-362-2764 AT LEAST TWO (2) WORKING DAYS BEFORE DIGGING OR OTHER EARTHWORK OPERATION.

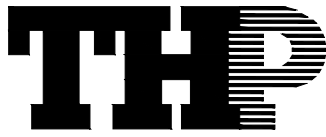
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STRUCTURAL GENERAL NOTES:

H. QUALITY CONTROL AND ASSURANCE

1. THE CONTRACTOR SHALL PERFORM QUALITY CONTROL, TESTING AND INSPECTION OF ALL WORK AS REQUIRED BY THE CONTRACT DOCUMENTS, INCLUDING REFERENCED CODES, SPECIFICATIONS AND STANDARDS.
2. THE OWNER WILL EMPLOY A TESTING AND INSPECTION AGENCY TO PERFORM SERVICES INDICATED TO BE BY THE OWNER IN THE PROJECT SPECIFICATIONS.
3. THE OWNER WILL ALSO EMPLOY QUALIFIED SPECIAL INSPECTORS TO PERFORM INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF THE BUILDING CODE. THE CONTRACTOR IS RESPONSIBLE FOR SCHEDULING ALL INSPECTIONS AND TESTS. ITEMS REQUIRING SPECIAL INSPECTION ON THIS PROJECT INCLUDE:
 - a. SOILS: ALL PILE INSTALLATION WORK.
 - b. CONCRETE: ALL CONCRETE WORK.
 - c. REINFORCING STEEL: ALL REINFORCING STEEL.

THE STRUCTURAL ENGINEER MAY GENERALLY OBSERVE THE PROGRESS OF THE WORK, BUT THEIR OBSERVATION SHALL NOT BE CONSTRUED AS INSPECTION.



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ALL DESIGN LOAD INFORMATION ON SKS-09 AND SKS-10 PROVIDED BY TK AIRPORT SOLUTIONS FOR GATE A3, AND SHOULD NOT BE ASSUMED APPLICABLE FOR OTHER LOCATIONS. VERIFY ALL LOADS FOR NEW FOUNDATIONS AS APPROPRIATE AT ALL NEW LOCATIONS.

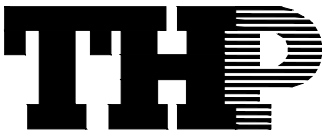
Inputs: Enter Fields Below For the Largest Bridge in the Customers Contract		Wind Load Calculation	Seismic Information	Walkway Information
Gate No.:	A3	Wind Speed, Operational: 65 mph	Spectral Response Accel (SS): 0.015	Is There A Walkway Attached? N
Bridge Model:	TB 43/2	Wind Speed, Stowed: 115 mph	Site Class (info from GC/client): C	
Live Load:	40.0 psf	Building Category: II	Site Coefficient (FA): 1.2	If Yes, Then Enter the Fields Below:
Roof Load:	25.0 psf	Exposure: C	Seismic Use Group: I	
Finished Floor Height:	12.1 ft		Design Spectral Response (SDS): 0.018	Walkway Length: 0 ft
Wind Load, Operational:	12.5 psf		Inverted Pendulum Structure:	
Wind Load, Stowed:	28.4 psf		Equivalent Lateral Force Procedure:	See Note 4.

Use LRFD Factored Loads? Y

Outputs: Bridge Model: TB 43/25.5-2 Steel		FORCE IN Z DIRECTION KIPS	MOMENT ABOUT Y KIP-FT	MOMENT ABOUT X KIP-FT	MOMENT ABOUT Z KIP-FT	FORCE IN Y DIRECTION KIPS
Operational:						
1.4D	[26.1]	[52.3]	[11.3]			
1.2D+1.6L+0.5R	[37.6]	[98.9]	[55.8]			
1.2D+1.6R+.5W	[32.7]	[82.9]	[132.6]	[9.0]	[2.5]	
1.2D+1.0W+1.0L+.5R	[33.1]	[83.1]	[284.3]	[18.1]	[5.1]	
0.9D+1.0W	[16.8]	[33.6]	[253.1]	[18.1]	[5.1]	
0.9D+1.0E	[16.8]	[33.7]	[7.7]	[0.1]	[0.0]	
Stowed:						
1.4D	[9.0]	[-32.0]	[11.3]			
1.2D+0.5R	[7.8]	[-30.7]	[9.7]			
1.2D+1.6R+.5W	[8.2]	[-37.7]	[193.2]	[-0.2]	[1.5]	
1.2D+1.0W+.5R	[7.8]	[-30.7]	[376.8]	[-0.4]	[3.1]	
0.9D+1.0W	[5.8]	[-20.6]	[374.4]	[-0.4]	[3.1]	
0.9D+1.0E	[5.8]	[-20.5]	[7.6]	[0.1]	[0.0]	

Notes:

- Calculations for reaction loads per International Building Code and the AISC LRFD.
- Rotunda column reactions vary more with design conditions than bridge model.
- Actual foundation design is by others. Appropriate safety factors should be applied to these loads.
- Walkway calculation is for a walkway that is supported on the terminal end by a column or support, and on the bridge end by a haunch. The WW is in-line with the bridge centerline.
- Reactions include auxiliary equipment (PCA, 400Hz) that may be on the bridge.
- Refer to SKS-10 for associated Rotunda Column Diagram to be utilized with load tables.



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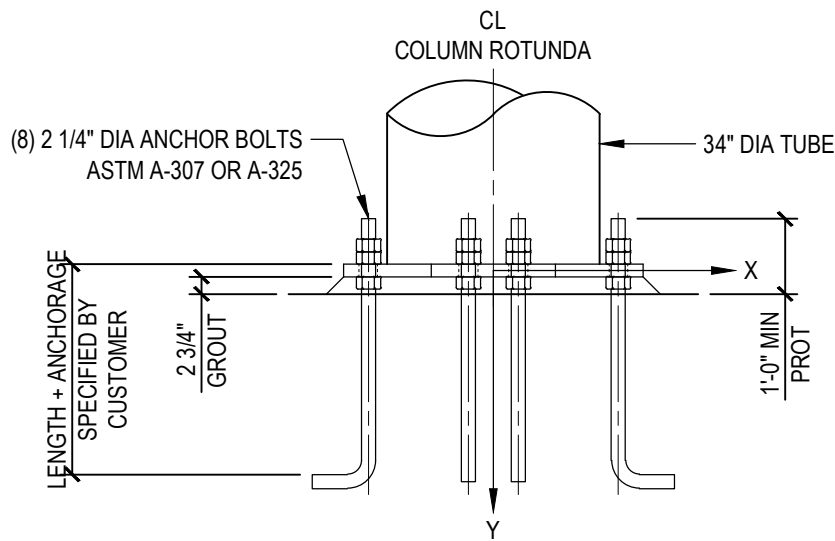
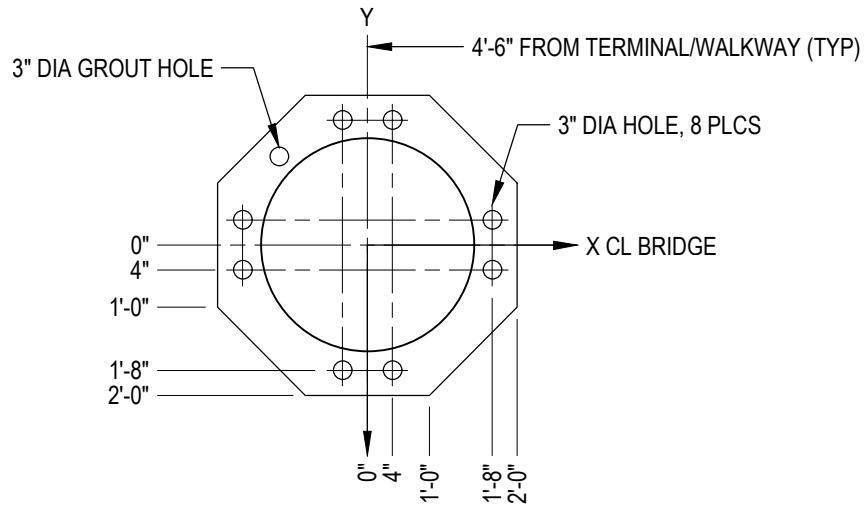
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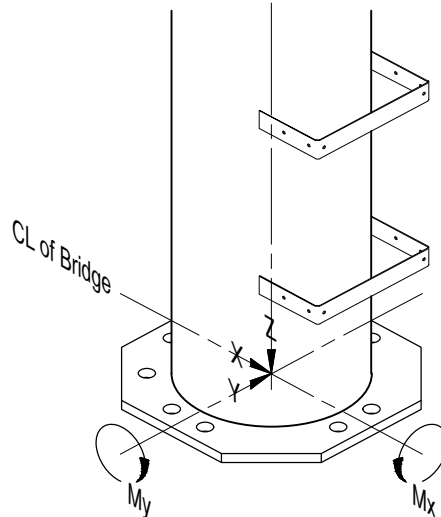
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NOTE: REFER TO SKS-09 FOR ASSOCIATED LOAD TABLES TO BE UTILIZED WITH ROTUNDA COLUMN DIAGRAMS.

PLAN VIEW, BASE PLATE



ALL DIMENSIONS IN INCHES



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